

**DIVISION: 03 00 00—CONCRETE**  
**Section: 03 16 00—Concrete Anchors**

**DIVISION: 05 00 00—METALS**  
**Section: 05 05 19—Post-Installed Concrete Anchors**

**REPORT HOLDER:**

**HILTI, INC.**

**EVALUATION SUBJECT:**

**HILTI HSL4 CARBON STEEL HEAVY DUTY EXPANSION ANCHORS FOR CRACKED AND UNCRACKED CONCRETE**

## 1.0 EVALUATION SCOPE

**Compliance with the following codes:**

- 2021, 2018, 2015, 2012, and 2009 *International Building Code*® (IBC)
- 2021, 2018, 2015, 2012, and 2009 *International Residential Code*® (IRC)
- 2013 *Abu Dhabi International Building Code* (ADIBC)<sup>†</sup>

<sup>†</sup>The ADIBC is based on the 2009 IBC. 2009 IBC code sections referenced in this report are the same sections in the ADIBC.

For evaluation for compliance with codes adopted by the Los Angeles Department of Building and Safety (LADBS), see [ESR-4386 LABC and LARC Supplement](#).

**Property evaluated:**

Structural

## 2.0 USES

The Hilti HSL4 carbon steel heavy duty expansion anchors are used as anchorage to resist static, wind, and seismic tension and shear loads in cracked and uncracked normal-weight and lightweight concrete having a specified compressive strength  $2,500 \text{ psi} \leq f'_c \leq 8,500 \text{ psi}$  ( $17.2 \text{ MPa} \leq f'_c \leq 58.6 \text{ MPa}$ ) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1].

The Hilti HSL4 anchors comply with Section 1901.3 of the 2021, 2018 and 2015 IBC, Section 1909 of the 2012 IBC, and Section 1912 of the 2009 IBC. The anchor system is an alternative to cast-in-place anchors described in Section 1908 of the 2012 IBC, and Section 1911 of the 2009 IBC. The anchors may also be used where an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

## 3.0 DESCRIPTION

### 3.1 HSL4 Carbon Steel Heavy Duty Sleeve Anchor:

**3.1.1 General:** The Hilti HSL4 Carbon Steel Heavy Duty Expansion Concrete Anchor, designated as the HSL4, is a torque-set, sleeve-type mechanical expansion anchor. The HSL4 is comprised of seven components which vary slightly according to anchor diameter, as shown in Figure 1 of this report. It is available in four head configurations, illustrated in Figure 2 of this report.

All carbon steel parts receive a minimum 5  $\mu\text{m}$  (0.0002 inch) thick galvanized zinc plating.

Dimensions and installation criteria are set forth in Tables 1 and 2 of this report. Application of torque at the head of the anchor causes the cone to be drawn into the expansion sleeve. This in turn causes the sleeve to expand against the wall of the drilled hole. The ribs on the collapsible element prevent rotation of the sleeve and cone during application of torque. Application of the specified installation torque induces a tension force in the bolt that is equilibrated by a precompression force in the concrete acting through the component being fastened. Telescopic deformation of the collapsible element prevents buildup of precompression in the anchor sleeve in cases where the shear sleeve is in contact with the washer, and permits the closure of gaps between the work surface and the component being fastened. Application of tension loads that exceed the precompression force in the bolt will cause the cone to displace further into the expansion sleeve (follow-up expansion), generating additional expansion force.

**3.1.2 HSL4 (Bolt):** The anchor consists of a stud bolt, steel washer, steel sleeve, collapsible plastic sleeve, steel expansion sleeve and steel cone. This anchor is available in carbon steel only. A product identification code is printed on the hex bolt of the anchor.

**3.1.3 HSL4-G (Stud):** The anchor has the same components and material specifications as the HSL4 (bolt) with the exception that the bolt is replaced by a carbon steel threaded rod and nut. A product identification code is printed on the hex nut of the anchor.

**3.1.4 HSL4-B (Torque-Limiting Nut):** The anchor has the same components and material specifications as the HSL4 (bolt) with the addition of a torque cap nut that permits the proper setting of the anchor without a torque-indicator wrench. When the anchor is tightened, the torque is transmitted to the cap. When the torque corresponding to the required anchor expansion is attained, the three countersunk rivets shear off. The torque cap nut breaks free exposing the permanent hex nut. A product identification code is printed on the bolt.

**3.1.5 HSL4-SK:** The anchor has the same components and material specifications as the HSL4 (bolt) except that the bolt head is configured for countersunk applications, is configured to accept a hexagonal Allen wrench and is provided with a conical washer.

### 3.2 Concrete:

Normal-weight and lightweight concrete must conform to Sections 1903 and 1905 of the IBC, as applicable.

## 4.0 DESIGN AND INSTALLATION

### 4.1 Strength Design:

**4.1.1 General:** Design strength of anchors complying with the 2021 IBC, as well as Section R301.1.3 of the 2021 IRC must be determined in accordance with ACI 318-19 Chapter 17 and this report.

Design strength of anchors complying with the 2018 and 2015 IBC, as well as Section R301.1.3 of the 2018 and 2015 IRC must be determined in accordance with ACI 318-14 Chapter 17 and this report.

Design strength of anchors complying with the 2012 IBC, and the 2012 IRC, must be in accordance with ACI 318-11 Appendix D and this report.

Design strength of anchors complying with the 2009 IBC and 2009 IRC must be in accordance with ACI 318-08 Appendix D and this report.

A Design example in accordance with the 2021, 2018, 2015 and 2012 IBC is shown in Figure 6 of this report.

Design parameters are based on the 2021 IBC (ACI 318-19), 2018 and 2015 IBC (ACI 318-14) and the 2012 IBC (ACI 318-11) unless noted otherwise in Sections 4.1.1 through 4.1.12 of this report. The strength design of anchors must comply with ACI 318-19 17.5.1.2, ACI 318-14 17.3.1 or ACI 318-11 D.4.1, as applicable, except as required in ACI 318-19 17.10, ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable. Strength reduction factors,  $\phi$ , as given in ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, must be used for load combinations calculated in accordance with Section 1605.1 of the 2021 IBC or Section 1605.2 of the 2018, 2015, 2012 and 2009 IBC and Section 5.3 of ACI 318 (-19 and -14) or Section 9.2 of ACI 318-11, as applicable. Strength reduction factors,  $\phi$ , as given in ACI 318-11 D.4.4 must be used for load combinations calculated in accordance with ACI 318-11 Appendix C.

The value of  $f'_c$  used in the calculations must be limited to a maximum of 8,000 psi (55.2 MPa), in accordance with ACI 318-19 17.3.1, ACI 318-14 17.2.7 or ACI 318-11 D.3.7, as applicable.

**4.1.2 Requirements for Static Steel Strength in Tension,  $N_{sa}$ :** The static steel strength in tension must be calculated in accordance with ACI 318-19 17.6.1.2, ACI 318-14 17.4.1.2 or ACI 318-11 D 5.1.2, as applicable. The values for  $N_{sa}$  are given in Table 2 of this report. Strength reduction factors,  $\phi$ , corresponding to ductile steel elements may be used for the HSL4.

**4.1.3 Requirements for Static Concrete Breakout Strength in Tension,  $N_{cb}$  and  $N_{cbg}$ :** The nominal concrete breakout strength of a single anchor or group of anchors in tension,  $N_{cb}$  and  $N_{cbg}$ , respectively must be calculated in accordance with ACI 318-19 17.6.2, ACI 318-14 17.4.2 or ACI 318-11 D.5.2, as applicable, with modifications as described in this section. The basic concrete breakout strength of a single anchor in tension,  $N_b$ , must be calculated in accordance with ACI 318-19 17.6.2.2, ACI 318-14 17.4.2.2 or ACI 318-11 D.5.2.2, as applicable, using the values of  $h_{ef,min}$  and  $K_{cr}$  as given in Table 2 of this report in

lieu of  $h_{ef}$  and  $k_c$ , respectively. The nominal concrete breakout strength in tension, in regions where analysis indicates no cracking in accordance with ACI 318-19 17.6.2.5.1, ACI 318-14 17.4.2.6 or ACI 318-11 D.5.2.6, as applicable, must be calculated with  $\Psi_{c,N} = 1.0$  and using the value of  $k_{uncr}$  as given in Table 2 of this report.

**4.1.4 Requirements for Static Pullout Strength in Tension,  $N_{pn}$ :** The nominal pullout strength of a single anchor, in accordance with ACI 318-19 17.6.3.1 and 17.6.3.2.1, ACI 318-14 17.4.3.1 and 17.4.3.2 or ACI 318-11 D.5.3.1 and D.5.3.2, as applicable, in cracked and uncracked concrete,  $N_{p,cr}$  and  $N_{p,uncr}$ , respectively, is given in Table 2 of this report. In lieu of ACI 318-19 17.6.3.3, ACI 318-14 17.4.3.6 or ACI 318-11 D.5.3.6, as applicable,  $\Psi_{c,P} = 1.0$  for all design cases. In accordance with ACI 318-19 17.6.3.2.1, ACI 318-14 17.4.3.2 or ACI 318-11 D.5.3.2, as applicable, the nominal pullout strength in cracked concrete must be adjusted by calculation according to the following equation:

$$N_{p,f'_c} = N_{p,cr} \sqrt{\frac{f'_c}{2,500}} \quad (\text{lb, psi}) \quad (\text{Eq-1})$$

$$N_{p,f'_c} = N_{p,cr} \sqrt{\frac{f'_c}{17.2}} \quad (\text{N, MPa})$$

In regions where analysis indicates no cracking in accordance with ACI 318-19 17.6.3.3, ACI 318-14 17.4.3.6 or ACI 318-11 D.5.3.6, as applicable, the nominal pullout strength in tension must be calculated according to the following equation for all anchors:

$$N_{p,f'_c} = N_{p,uncr} \sqrt{\frac{f'_c}{2,500}} \quad (\text{lb, psi}) \quad (\text{Eq-2})$$

$$N_{p,f'_c} = N_{p,uncr} \sqrt{\frac{f'_c}{17.2}} \quad (\text{N, MPa})$$

Where values for  $N_{p,cr}$  or  $N_{p,uncr}$  are not provided in Table 2, the pullout strength in tension need not be evaluated.

**4.1.5 Requirements for Static Steel Strength in Shear,  $V_{sa}$ :** The nominal steel strength in shear,  $V_{sa}$ , in accordance with ACI 318-19 17.7.1.2, ACI 318-14 17.5.1.2 or ACI 318-11 D.6.1.2, as applicable, is given in Table 2 of this report must be used in lieu of the value derived by calculation from ACI 318-19 Eq. 17.7.1.2b, ACI 318-14 Eq. 17.5.1.2b or ACI 318-11, Eq D-29, as applicable. Strength reduction factors,  $\phi$ , corresponding to ductile steel elements may be used for the HSL4.

**4.1.6 Requirements for Static Concrete Breakout Strength in Shear,  $V_{cb}$  or  $V_{cbg}$ :** The nominal concrete breakout strength in shear of a single anchor or group of anchors,  $V_{cb}$  or  $V_{cbg}$ , respectively, must be calculated in accordance with ACI 318-19 17.7.2, ACI 318-14 17.5.2 or ACI 318-11 D.6.2, as applicable, with modifications as provided in this section. The basic concrete breakout strength of a single anchor in shear,  $V_b$ , must be calculated in accordance with ACI 318-19 17.7.2.2.1, ACI 318-14 17.5.2.2 or ACI 318-11 D.6.2.2, as applicable, using the values of  $l_e$  and  $d_a$  given in Table 2 of this report.

**4.1.7 Requirements for Static Concrete Pryout Strength in Shear,  $V_{cp}$  or  $V_{cpg}$ :** The nominal static concrete pryout strength of a single anchor or group of anchors in shear,  $V_{cp}$  or  $V_{cpg}$ , must be calculated in accordance with ACI 318-19 17.7.3, ACI 318-14 17.5.3 or ACI 318-11 D.6.3, as applicable, modified by using the value of  $k_{cp}$  provided in Table 2 of this report and the value of  $N_{cb}$  or  $N_{cbg}$  as calculated in accordance with Section 4.1.3 of this report.

### 4.1.8 Requirements for Seismic Design:

**4.1.8.1 General:** For load combinations including seismic, the design must be performed in accordance with ACI

318-19 17.10, ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable. Modifications to ACI 318-19 17.10 or ACI 318-14 17.2.3 shall be applied under Section 1905.1.8 of the 2021, 2018, or 2015 IBC, as applicable. For the 2012 IBC, Section 1905.1.9 shall be omitted. Modifications to ACI 318-08 D.3.3 shall be applied under Section 1908.1.9 of the 2009 IBC.

**4.1.8.2 Seismic Tension:** The nominal steel strength and the nominal concrete breakout strength for anchors in tension must be calculated according to ACI 318-19 17.6.1 and 17.6.2, ACI 318-14 17.4.1 and 17.4.2 or ACI 318-11 D.5.1 and D.5.2, respectively, as applicable, as described in Sections 4.1.2 and 4.1.3 of this report. In accordance with ACI 318-19 17.6.3.2.1, ACI 318-14 17.4.3.2 or ACI 318-11 D.5.3.2, as applicable, the appropriate pullout strength in tension for seismic loads,  $N_{p,eq}$ , described in Table 2 must be used in lieu of  $N_p$ . The value of  $N_{p,eq}$  may be adjusted by calculation for concrete strength in accordance with Eq-1 and Section 4.1.4 whereby the value of  $N_{p,eq}$  must be substituted for  $N_{p,cr}$ . If no values for  $N_{p,eq}$  are given in Table 2, the static design strength values govern.

**4.1.8.3 Seismic Shear:** The nominal concrete breakout strength and pryout strength for anchors in shear must be calculated according to ACI 318-19 17.7.2 and 17.7.3, ACI 318-14 17.5.2 and 17.5.3 or ACI 318-11 D.6.2 and D.6.3, respectively, as applicable, as described in Sections 4.1.6 and 4.1.7 of this report. In accordance with ACI 318-19 17.7.1.2, ACI 318-14 17.5.1.2 or ACI 318-11 D.6.1.2, as applicable, the appropriate value for nominal steel strength for seismic loads,  $V_{sa,eq}$  described in Table 2 must be used in lieu of  $V_{sa}$ .

**4.1.9 Requirements for Interaction of Tensile and Shear Forces:** For anchors or groups of anchors that are subject to the effects of combined tensile and shear forces, the design must be performed in accordance with ACI 318-19 17.8, ACI 318-14 17.6 or ACI 318-11 D.7, as applicable.

**4.1.10 Requirements for Critical Edge Distance:** In applications where  $c < C_{ac}$  and supplemental reinforcement to control splitting of the concrete is not present, the concrete breakout strength in tension for uncracked concrete, calculated according to ACI 318-19 17.6.2, ACI 318-14 17.4.2 or ACI 318-11 D.5.2, as applicable, must be further multiplied by the factor  $\Psi_{cp,N}$  as given by the following equation:

$$\Psi_{cp,N} = \frac{c}{C_{ac}} \tag{Eq-4}$$

where the factor  $\Psi_{cp,N}$  need not be taken as less than  $\frac{1.5 h_{ef}}{C_{ac}}$ . For all other cases,  $\Psi_{cp,N} = 1.0$ . In lieu of ACI

318-19 17.9.5, ACI 318-14 17.7.6 or ACI 318-11 D.8.6, as applicable, values for the critical edge distance  $C_{ac}$  must be taken from Table 3 of this report. For the HSL4 carbon steel anchors, the values  $C_{ac,A}$  are valid for a member thickness  $h \geq h_{min,A}$  and the values  $C_{ac,B}$  for  $h_{min,B} \leq h < h_{min,A}$ .

**4.1.11 Requirements for Minimum Member Thickness, Minimum Anchor Spacing and Minimum Edge Distance:** In lieu of ACI 318-19 17.9.2, ACI 318-14 17.7.1 and 17.7.3 or ACI 318-11 D.8.1 and D.8.3, respectively, as applicable, values of  $s_{min}$  and  $c_{min}$  as given in Table 3 of this report must be used. In lieu of ACI 318-19 17.9.4, ACI 318-14 17.7.5 or ACI 318-11 D.8.5, as applicable, minimum member thicknesses  $h_{min}$  as given in Table 3 of this report must be used. Additional combinations for minimum edge distance  $C_{min}$  and spacing  $s_{min}$  may be derived by linear interpolation

between the given boundary values. (See example in Table 3 and Figure 4 of this report.)

**Lightweight Concrete:** For the use of anchors in lightweight concrete, the modification factor  $\lambda_a$  equal to 0.8 $\lambda$  is applied to all values of  $\sqrt{f'_c}$  affecting  $N_n$  and  $V_n$ .

For ACI 318-19 (2021 IBC), ACI 318-14 (2018 and 2015 IBC), ACI 318-11 (2012 IBC) and ACI 318-08 (2009 IBC),  $\lambda$  shall be determined in accordance with the corresponding version of ACI 318.

**4.2 Allowable Stress Design (ASD):**

**General:**

**4.2.1** Design values for use with allowable stress design load combinations calculated in accordance with Section 1605.3 of the IBC shall be established as follows:

$$T_{allowable,ASD} = \frac{\phi N_n}{\alpha} \tag{Eq-5}$$

$$V_{allowable,ASD} = \frac{\phi V_n}{\alpha} \tag{Eq-6}$$

where

$T_{allowable,ASD}$  = Allowable tension load (lbf or kN)

$V_{allowable,ASD}$  = Allowable shear load (lbf or kN)

$N_n$  = Lowest design strength of an anchor or anchor group in tension as determined in accordance with ACI 318 (-19 and -14) Chapter 17 and 2021, 2018 and 2015 IBC Section 1905.1.8, ACI 318-11 Appendix D, ACI 318-08 Appendix D and 2009 IBC Section 1908.1.9, and Section 4.1 of this report, as applicable. For 2012 IBC, Section 1905.1.19 shall be omitted.

$V_n$  = Lowest design strength of an anchor or anchor group in shear as determined in accordance with ACI 318 (-19 and -14) Chapter 17 and 2021, 2018 and 2015 IBC Section 1905.1.8, ACI 318-11 Appendix D, ACI 318-08 Appendix D and 2009 IBC Section 1908.1.9, and Section 4.1 of this report, as applicable. For 2012 IBC, Section 1905.1.9 shall be omitted.

$\alpha$  = Conversion factor calculated as a weighted average of the load factors for the controlling load combination. In addition,  $\alpha$  shall include all applicable factors to account for nonductile failure modes and required over-strength.

The requirements for member thickness, edge distance and spacing, described in this report, must apply. An example of allowable stress design values for illustrative purposes in shown in Table 4.

**4.2.2 Requirements for Interaction of Tensile and Shear Forces:** The interaction must be calculated and consistent with ACI 318-19 17.8, ACI 318-14 17.6 or ACI 318 (-11, -08) D.7, as applicable, as follows:

For shear loads  $V_{applied} \leq 0.2V_{allowable,ASD}$ , the full allowable load in tension  $T_{allowable,ASD}$  may be taken.

For tension loads  $T_{applied} \leq 0.2T_{allowable,ASD}$ , the full allowable load in shear  $V_{allowable,ASD}$  may be taken.

For all other cases:

$$\frac{T_{applied}}{T_{allowable,ASD}} + \frac{V_{applied}}{V_{allowable,ASD}} \leq 1.2 \tag{Eq-7}$$

### 4.3 Installation:

Installation parameters are provided in Table 1 and in Figure 3 of this report. Anchors must be installed per the manufacturer's printed installation instructions, as depicted in Figure 5, and this report. Anchor locations must comply with this report and the plans and specifications approved by the code official. Anchors must be installed in holes drilled into concrete using carbide-tipped drill bits complying with ANSI B212.15-1994 or using the Hilti SafeSet System™ with Hilti TE-YD or TE-CD Hollow Drill Bits complying with ANSI B212.15-1994 with a Hilti vacuum with a minimum value for the maximum volumetric flow rate of 129 CFM (61  $\ell/s$ ). Alternatively, HSL4 carbon steel anchors (all variants) may be installed in holes drilled using SPX-T core bits (with the DD-30 or DD-EC-1 coring tools) or SPX-H, SPX-L or SPX-L Handheld core bits (with the DD-110 to DD-250 coring tools). Prior to anchor installation, the hole must be cleaned in accordance with the manufacturer's published installation instructions. The nut must be tightened against the washer until the torque values,  $T_{inst}$ , specified in Table 1 are achieved. HSL4 and HSL4-G anchors with diameters of M8 through M16 may be tightened using the Hilti AT Tool in accordance with Figure 5.

### 4.4 Special Inspection:

Periodic special inspection is required, in accordance with Section 1705.1.1 and Table 1705.3 of the 2021, 2018 and 2015 IBC and 2012 IBC; Section 1704.15 and Table 1704.4 of the 2009 IBC, as applicable. The special inspector must make periodic inspections during anchor installation to verify anchor type, anchor dimensions, concrete type, concrete compressive strength, hole dimensions, anchor spacing, edge distances, concrete thickness, anchor embedment, installation torque, and adherence to the manufacturer's published installation instructions. The special inspector must be present as often as required in accordance with the "statement of special inspection". Under the IBC, additional requirements as set forth in Sections 1705, 1706 and 1707 must be observed, where applicable.

## 5.0 CONDITIONS OF USE

The Hilti HSL4 anchors described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1 Anchor sizes, dimensions and minimum embedment depths are as set forth in the tables of this report.
- 5.2 The anchors must be installed in accordance with the manufacturer's published installation instructions and this report, in cracked and uncracked normal-weight and lightweight concrete having a specified compressive strength of  $f'_c = 2,500$  psi to 8,500 psi (17.2 MPa to 58.6 MPa) [minimum of 24 MPa is required under ADIBC Appendix L, Section 5.1.1]. In case of conflict between this report and the manufacturer's instructions, this report governs.
- 5.3 The values of  $f'_c$  used for calculation purposes must not exceed 8,000 psi (55.1 MPa).
- 5.4 The concrete shall have attained its minimum design strength prior to installation of the anchors.
- 5.5 Strength design values are established in accordance with Section 4.1 of this report.
- 5.6 Allowable stress design values are established in accordance with Section 4.2 of this report.
- 5.7 Anchor spacing and edge distance as well as minimum member thickness must comply with Table 3 of this report.

5.8 Prior to installation, calculations and details demonstrating compliance with this report must be submitted to the code official. The calculations and details must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.

5.9 Since an ICC-ES acceptance criteria for evaluating data to determine the performance of expansion anchors subjected to fatigue or shock loading is unavailable at this time, the use of these anchors under such conditions is beyond the scope of this report.

5.10 Anchors may be installed in regions of concrete where cracking has occurred or where analysis indicates cracking may occur ( $f_t > f_r$ ), subject to the conditions of this report.

5.11 Anchors may be used to resist short-term loading due to wind or seismic forces, subject to the conditions of this report.

5.12 Where not otherwise prohibited in the code, anchors are permitted for use with fire-resistance-rated construction provided that at least one of the following conditions is fulfilled:

- Anchors are used to resist wind or seismic forces only.
- Anchors that support a fire-resistance-rated envelope or a fire-resistance-rated membrane, are protected by approved fire-resistance-rated materials, or have been evaluated for resistance to fire exposure in accordance with recognized standards.
- Anchors are used to support nonstructural elements.

5.13 Use of zinc-coated carbon steel anchors is limited to dry, interior locations.

5.14 Special inspection must be provided in accordance with Section 4.4 of this report.

5.15 Anchors are manufactured for Hilti, Inc., under an approved quality control program with inspections by ICC-ES.

## 6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Mechanical Anchors in Concrete Elements (AC193), dated October 2017 (editorially revised December 2020), which incorporates requirements in ACI 355.2-19 and ACI 355.2-07, for use in cracked and uncracked concrete; and quality control documentation.

## 7.0 IDENTIFICATION

7.1 The anchors are identified by packaging labeled with the evaluation report holder's name (Hilti, Inc.) and address, anchor name, anchor size, evaluation report number (ESR-4386). The anchors have the letters HSL4 and the anchor size embossed on the sleeve.

7.2 The report holder's contact information is as follows:

**HILTI, INC.**  
**7250 DALLAS PARKWAY, SUITE 1000**  
**PLANO, TEXAS 75024**  
**(800) 879-8000**  
[www.hilti.com](http://www.hilti.com)

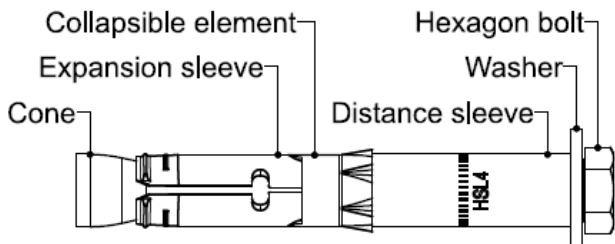


FIGURE 1—COMPONENTS OF THE HSL4 (BOLT VERSION SHOWN)

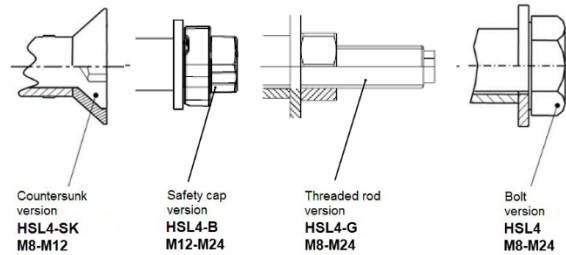


FIGURE 2—HEAD STYLES OF THE HSL4

TABLE 1—SETTING INFORMATION<sup>2</sup>

Setting Information		Symbol	Units	Nominal anchor diameter					
				M8	M10	M12	M16	M20	M24
Nominal drill bit or core bit diameter <sup>1</sup>		$d_{bit}$	mm	12	15	18	24	28	32
Minimum hole depth	HSL4, HSL4-G, HSL4-B, HSL4-SK	$h_{hole}$	mm (in.)	80 (3.15)	90 (3.54)	105 (4.13)	125 (4.92)	155 (6.10)	180 (7.09)
Clearance hole diameter in part being fastened		$d_f$	mm (in.)	14 (0.55)	17 (0.67)	20 (0.79)	26 (1.02)	31 (1.22)	35 (1.38)
Max. cumulative gap between part(s) being fastened and concrete surface		-	mm (in.)	4 (0.16)	5 (0.20)	8 (0.31)	9 (0.35)	12 (0.47)	16 (0.63)
Washer diameter HSL4, HSL4-G, HSL4-B		$d_w$	mm (in.)	20 (0.79)	25 (0.98)	30 (1.18)	40 (1.57)	45 (1.77)	50 (1.97)
Installation torque	HSL4	$T_{inst}$	Nm (ft-lb)	15 (11)	25 (18)	60 (44)	75 (55)	145 (107)	210 (155)
	HSL4-G		Nm (ft-lb)	20 (15)	27 (20)	60 (44)	70 (52)	105 (77)	180 (132)
	HSL4-SK		Nm (ft-lb)	25 (18)	32 (24)	65 (48)	N/A	N/A	N/A
Wrench size	HSL4, HSL4-G	SW	mm	13	17	19	24	30	36
	HSL4-B	SW	mm	N/A	N/A	24	30	36	41
Allen wrench size	HSL4-SK	SW	mm	5	6	8	N/A	N/A	N/A
Diameter of countersunk hole HSL4-SK		$d_{sk}$	mm (in.)	22.5 (0.89)	25.5 (1.00)	32.9 (1.29)	N/A	N/A	N/A

For pound-inch units: 1 mm = 0.03937 inches, 1 Nm = 0.7376 ft-lbf.

<sup>1</sup>Use metric bits only.

<sup>2</sup>N/A = Not Available

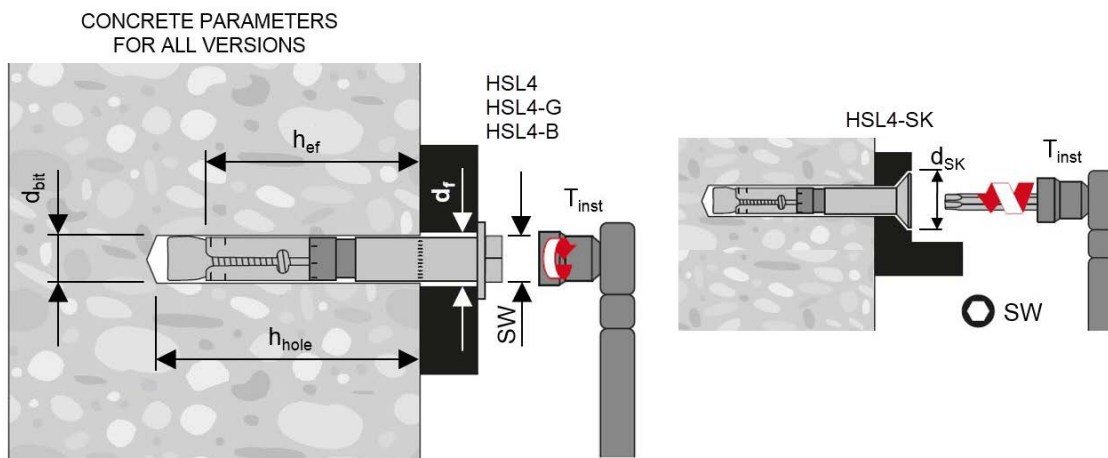


FIGURE 3—HSL4 IN THE INSTALLED CONDITION

TABLE 2—DESIGN INFORMATION FOR CARBON STEEL HSL4 (ALL VERSIONS)

Design parameter	Symbol	Units	Nominal anchor diameter					
			M8	M10	M12	M16	M20	M24
Anchor O.D.	$d_a$	mm in.	12 (0.47)	15 (0.59)	18 (0.71)	24 (0.94)	28 (1.10)	32 (1.26)
Effective min. embedment depth <sup>1</sup>	$h_{ef,min}$	mm in.	60 (2.36)	70 (2.76)	80 (3.15)	100 (3.94)	125 (4.92)	150 (5.91)
Anchor category <sup>2</sup>	1,2 or 3	-	1	1	1	1	1	1
Strength reduction factor for tension, steel failure modes <sup>3</sup>	$\phi$	-	0.75					
Strength reduction factor for shear, steel failure modes <sup>3</sup>	$\phi$	-	0.65					
Strength reduction factor for tension, concrete failure modes <sup>3</sup>	$\phi$	Cond.A	0.75					
		Cond.B	0.65					
Strength reduction factor for shear, concrete failure modes <sup>3</sup>	$\phi$	Cond.A	0.75					
		Cond.B	0.70					
Yield strength of anchor steel	$f_{ya}$	lb/in <sup>2</sup> (N/mm <sup>2</sup> )	92,800 (640.0)					
Ultimate strength of anchor steel	$f_{uta}$	lb/in <sup>2</sup> (N/mm <sup>2</sup> )	116,000 (800.0)					
Tensile stress area	$A_{se,N}$	in <sup>2</sup> (mm <sup>2</sup> )	0.057 (36.6)	0.090 (58.0)	0.131 (84.3)	0.243 (157.0)	0.380 (245.0)	0.547 (353.0)
Steel strength in tension	$N_{sa}$	lb (kN)	6,612 (29.4)	10,440 (46.4)	15,196 (67.6)	28,188 (125.4)	44,080 (196.1)	63,452 (282.2)
Effectiveness factor uncracked concrete	$k_{uncr}$	- (SI)	24 (10)	24 (10)	24 (10)	24 (10)	24 (10)	24 (10)
Effectiveness factor cracked concrete <sup>4</sup>	$k_{cr}$	- (SI)	17 (7.1)	24 (10)	24 (10)	24 (10)	24 (10)	24 (10)
Modification factor for cracked and uncracked concrete <sup>5</sup>	$\psi_{C,N}$	-	1.00	1.00	1.00	1.00	1.00	1.00
Pullout strength uncracked concrete <sup>6</sup>	$N_{p,uncr}$	lb (kN)	4,204 (18.7)	N/A	N/A	N/A	N/A	N/A
Pullout strength cracked concrete <sup>6</sup>	$N_{p,cr}$	lb (kN)	2,810 (12.5)	4,496 (20.0)	N/A	N/A	N/A	N/A
Steel strength in shear HSL4,-B,-SK	$V_{sa}$	lb (kN)	7,239 (32.2)	10,229 (45.5)	14,725 (65.5)	26,707 (118.8)	39,521 (175.8)	45,951 (204.4)
Steel strength in shear HSL4-G		lb (kN)	6,070 (27.0)	8,385 (37.3)	12,162 (54.1)	22,683 (100.9)	33,159 (147.5)	43,169 (192.0)
Coefficient for pryout strength	$k_{cp}$	-	1.0	2.0				
Load bearing length of anchor in shear	$\ell_e$	mm (in.)	24 (0.94)	30 (1.18)	36 (1.42)	48 (1.89)	56 (2.20)	64 (2.52)
Tension pullout strength seismic <sup>7</sup> HSL4,-B,-SK	$N_{p,eq}$	lb (kN)	2,810 (12.5)	4,496 (20.0)	N/A	N/A	N/A	14,320 (63.7)
Tension pullout strength seismic <sup>7</sup> HSL4-G		lb (kN)	2,810 (12.5)	4,496 (20.0)	N/A	N/A	N/A	N/A
Steel strength in shear, seismic <sup>7</sup> HSL4,-B,-SK	$V_{sa,eq}$	lb (kN)	4,609 (20.5)	8,453 (37.6)	11,892 (52.9)	24,796 (110.3)	29,135 (129.6)	38,173 (169.8)
Steel strength in shear, seismic <sup>7</sup> HSL4-G		lb (kN)	3,777 (16.8)	6,924 (30.8)	9,824 (43.7)	21,065 (93.7)	24,459 (108.8)	N/A
Axial stiffness in service load range <sup>8</sup>	uncracked concrete	$\beta_{uncr}$	10 <sup>3</sup> lb/in.	300				
	cracked concrete	$\beta_{cr}$		30	70	130	130	130

For SI: 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 0.006895 MPa. For pound-inch units: 1 mm = 0.03937 inches.

<sup>1</sup>See Table 1.

<sup>2</sup>See ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable.

<sup>3</sup>The strength reduction factor applies when the load combinations from the IBC or ACI 318 are used and the requirements of ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate strength reduction factor must be determined in accordance with ACI 318-11 D.4.4.

<sup>4</sup>See ACI 318-19 17.6.2.2, ACI 318-14 17.4.2.2 or ACI 318-11 D.5.2.2, as applicable.

<sup>5</sup>See ACI 318-19 17.6.2.5.1, ACI 318-14 17.4.2.6 or ACI 318-11 D.5.2.6, as applicable.

<sup>6</sup>NA (not applicable) denotes that this value does not control for design. See Section 4.1.4 of this report.

<sup>7</sup>NA (not applicable) denotes that this value does not control for design. See Section 4.1.8 of this report.

<sup>8</sup>Minimum axial stiffness values, maximum values may be 3 times larger (e.g., due to high-strength concrete).

TABLE 3—HSL4 EDGE DISTANCE, SPACING AND MEMBER THICKNESS REQUIREMENTS<sup>1, 2</sup>

Case	Dimensional parameter	Symbol	Units	Nominal anchor diameter					
				M8	M10	M12	M16	M20	M24
A	Minimum concrete thickness	$h_{min,A}$	in. (mm)	4 <sup>3</sup> / <sub>4</sub> (120)	5 <sup>1</sup> / <sub>2</sub> (140)	6 <sup>1</sup> / <sub>4</sub> (160)	7 <sup>7</sup> / <sub>8</sub> (200)	9 <sup>7</sup> / <sub>8</sub> (250)	11 <sup>7</sup> / <sub>8</sub> (300)
A	Critical edge distance <sup>2</sup>	$C_{ac,A}$	in. (mm)	4 <sup>3</sup> / <sub>8</sub> (110)	4 <sup>3</sup> / <sub>8</sub> (110)	4 <sup>3</sup> / <sub>4</sub> (120)	5 <sup>7</sup> / <sub>8</sub> (150)	8 <sup>7</sup> / <sub>8</sub> (225)	8 <sup>7</sup> / <sub>8</sub> (225)
A	Minimum edge distance <sup>3</sup>	$C_{min,AA}$	in. (mm)	2 <sup>3</sup> / <sub>8</sub> (60)	2 <sup>3</sup> / <sub>4</sub> (70)	3 <sup>1</sup> / <sub>2</sub> (90)	4 <sup>3</sup> / <sub>4</sub> (120)	5 (125)	5 <sup>7</sup> / <sub>8</sub> (150)
A	Minimum anchor spacing <sup>3</sup>	$S_{min,AA}$	in. (mm)	5 <sup>1</sup> / <sub>2</sub> (140)	9 <sup>1</sup> / <sub>2</sub> (240)	11 (280)	12 <sup>5</sup> / <sub>8</sub> (320)	13 <sup>3</sup> / <sub>4</sub> (350)	11 <sup>7</sup> / <sub>8</sub> (300)
A	Minimum edge distance <sup>3</sup>	$C_{min,AB}$	in. (mm)	3 <sup>3</sup> / <sub>8</sub> (85)	5 (125)	6 <sup>1</sup> / <sub>8</sub> (155)	7 <sup>7</sup> / <sub>8</sub> (200)	8 <sup>1</sup> / <sub>4</sub> (210)	8 <sup>1</sup> / <sub>4</sub> (210)
A	Minimum anchor spacing <sup>3</sup>	$S_{min,AB}$	in. (mm)	2 <sup>3</sup> / <sub>8</sub> (60)	2 <sup>3</sup> / <sub>4</sub> (70)	3 <sup>1</sup> / <sub>8</sub> (80)	4 (100)	5 (125)	5 <sup>7</sup> / <sub>8</sub> (150)
B	Minimum concrete thickness	$h_{min,B}$	in. (mm)	4 <sup>3</sup> / <sub>8</sub> (110)	4 <sup>3</sup> / <sub>4</sub> (120)	5 <sup>3</sup> / <sub>8</sub> (135)	6 <sup>1</sup> / <sub>4</sub> (160)	7 <sup>1</sup> / <sub>2</sub> (190)	8 <sup>7</sup> / <sub>8</sub> (225)
B	Critical edge distance <sup>2</sup>	$C_{ac,B}$	in. (mm)	5 <sup>7</sup> / <sub>8</sub> (150)	6 <sup>7</sup> / <sub>8</sub> (175)	7 <sup>7</sup> / <sub>8</sub> (200)	9 <sup>7</sup> / <sub>8</sub> (250)	12 <sup>3</sup> / <sub>8</sub> (312.5)	14 <sup>3</sup> / <sub>4</sub> (375)
B	Minimum edge distance <sup>3</sup>	$C_{min,BA}$	in. (mm)	2 <sup>3</sup> / <sub>8</sub> (60)	3 <sup>1</sup> / <sub>2</sub> (90)	4 <sup>3</sup> / <sub>8</sub> (110)	6 <sup>1</sup> / <sub>4</sub> (160)	7 <sup>7</sup> / <sub>8</sub> (200)	8 <sup>7</sup> / <sub>8</sub> (225)
B	Minimum anchor spacing <sup>3</sup>	$S_{min,BA}$	in. (mm)	7 (180)	10 <sup>1</sup> / <sub>4</sub> (260)	12 <sup>5</sup> / <sub>8</sub> (320)	15 (380)	15 <sup>3</sup> / <sub>4</sub> (400)	15 (380)
B	Minimum edge distance <sup>3</sup>	$C_{min,BB}$	in. (mm)	4 (100)	6 <sup>1</sup> / <sub>4</sub> (160)	7 <sup>7</sup> / <sub>8</sub> (200)	10 <sup>5</sup> / <sub>8</sub> (270)	11 <sup>7</sup> / <sub>8</sub> (300)	12 <sup>5</sup> / <sub>8</sub> (320)
B	Minimum anchor spacing <sup>3</sup>	$S_{min,BB}$	in. (mm)	2 <sup>3</sup> / <sub>8</sub> (60)	2 <sup>3</sup> / <sub>4</sub> (70)	3 <sup>1</sup> / <sub>8</sub> (80)	4 (100)	5 (125)	5 <sup>7</sup> / <sub>8</sub> (150)

For SI: 1 inch = 25.4 mm.

<sup>1</sup>See Section 4.1.10 of this report.

<sup>2</sup>See Section 4.1.11 of this report.

<sup>3</sup>Denotes admissible combinations of  $h_{min}$ ,  $C_{ac}$ ,  $C_{min}$  and  $S_{min}$ . For example,  $h_{min,A} + C_{ac,A} + C_{min,AA} + S_{min,AA}$  or  $h_{min,A} + C_{ac,A} + C_{min,AB} + S_{min,AB}$  are admissible, but  $h_{min,A} + C_{ac,B} + C_{min,AB} + S_{min,BB}$  is not. However, other admissible combinations for minimum edge distance  $C_{min}$  and spacing  $S_{min}$  for  $h_{min,A}$  or  $h_{min,B}$  may be derived by linear interpolation between boundary values (see example for  $h_{min,A}$  below).

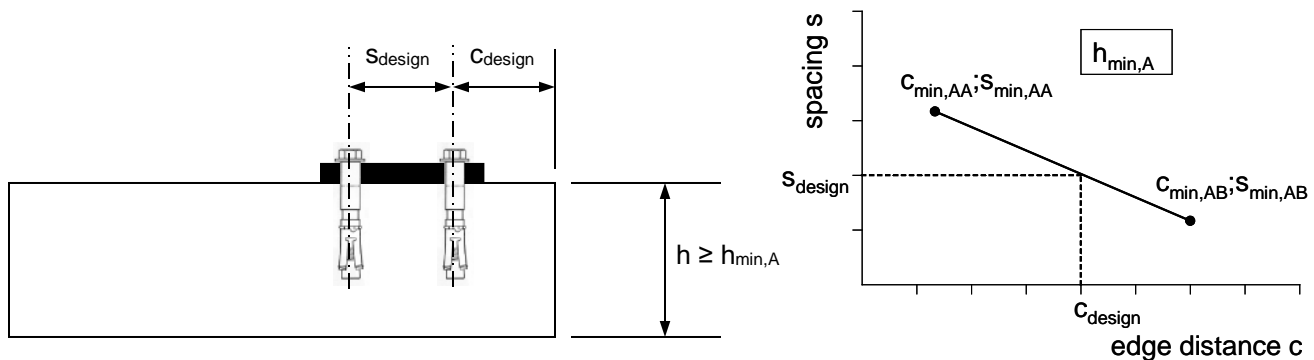


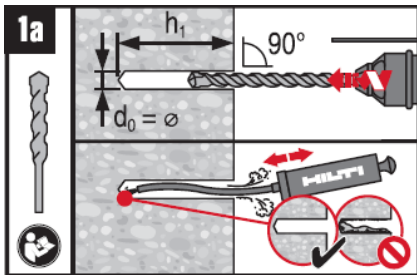
FIGURE 4—EXAMPLE OF ALLOWABLE INTERPOLATION OF MINIMUM EDGE DISTANCE AND MINIMUM SPACING

TABLE 4—EXAMPLE ALLOWABLE STRESS DESIGN VALUES FOR ILLUSTRATIVE PURPOSES<sup>1,2,3,4,5,6,7,8,9,10</sup>

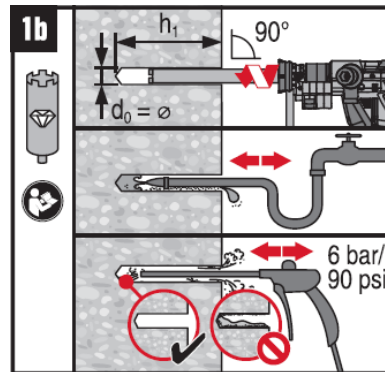
Nominal Anchor Diameter	Effective Embedment		Allowable Tension (lbs) $f'_c = 2500 \text{ psi}$
	mm	inches	
M8	60	2.36	1,846
M10	70	2.76	2,417
M12	80	3.15	2,946
M16	100	3.94	4,122
M20	125	4.92	5,751
M24	150	5.91	7,572

<sup>1</sup>Single anchor with static tension load only.  
<sup>2</sup>Concrete determined to remain uncracked for the life of the anchorage.  
<sup>3</sup>Load combinations from ACI 318 (-19 and -14) Section 5.3 or ACI 318-11 Section 9.2, as applicable (no seismic loading).  
<sup>4</sup>30% dead load and 70% live load, controlling load combination  $1.2D + 1.6L$   
<sup>5</sup>Calculation of weighted average for  $\alpha = 0.3 \cdot 1.2 + 0.7 \cdot 1.6 = 1.48$   
<sup>6</sup> $f'_c = 2,500 \text{ psi}$  (normal weight concrete).  
<sup>7</sup> $C_{a1} = C_{a2} \geq C_{ac}$   
<sup>8</sup> $h \geq h_{min}$   
<sup>9</sup>Values are for Condition B where supplementary reinforcement in accordance with ACI 318-19 17.5.3, ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c) is not provided, as applicable.  
<sup>10</sup> $\phi$  factor is 0.65.

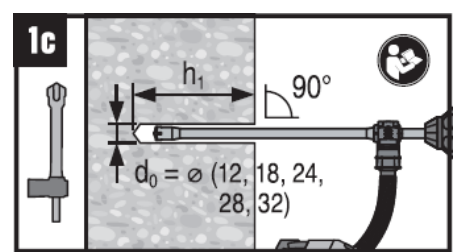
HSL4 carbon steel anchors with standard carbide drill bit



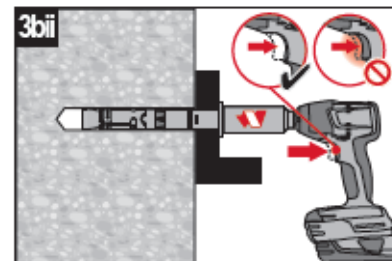
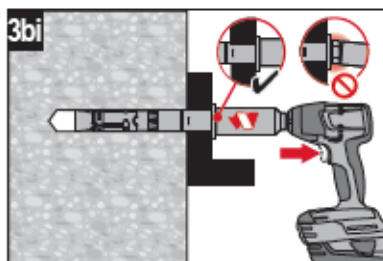
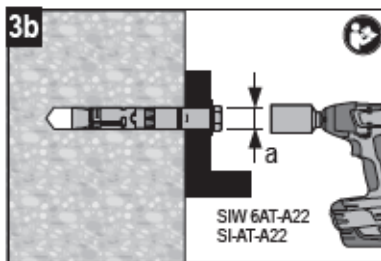
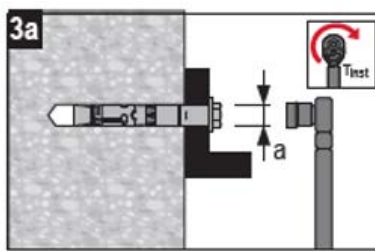
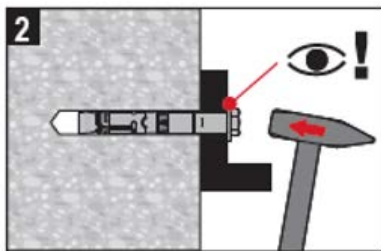
HSL4 carbon steel anchors with diamond core drilling



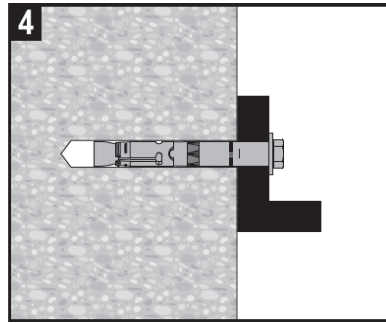
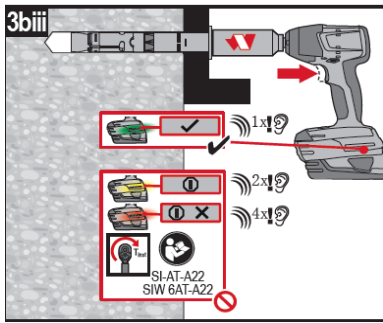
HSL4 carbon steel anchors with hollow drill bit



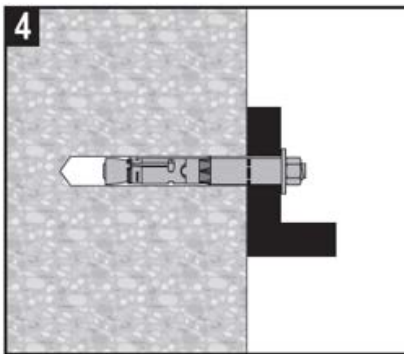
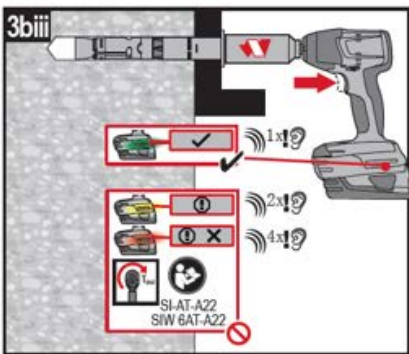
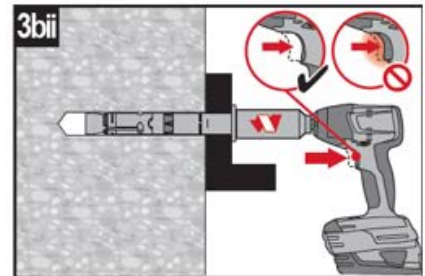
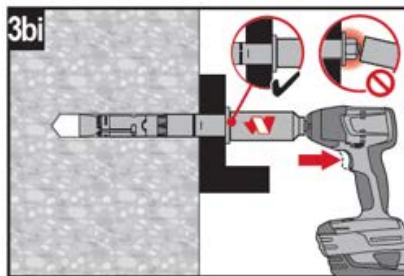
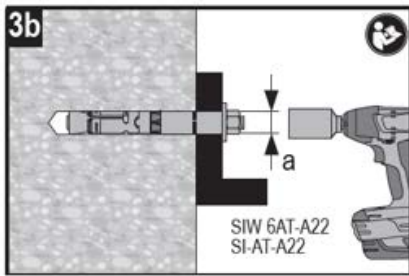
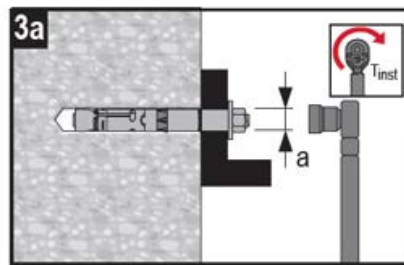
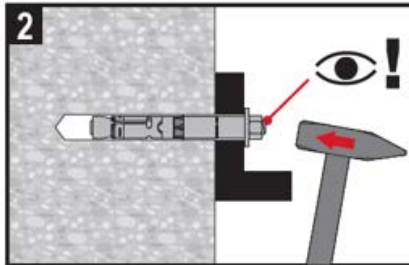
HSL4



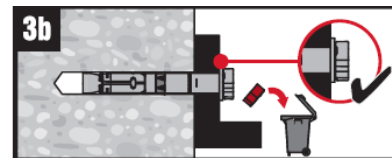
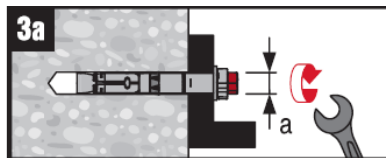
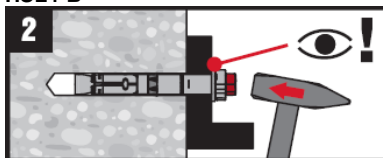




HSL4-G



HSL4-B



HSL4-SK

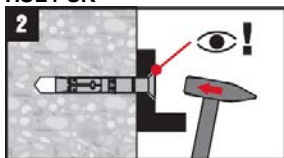
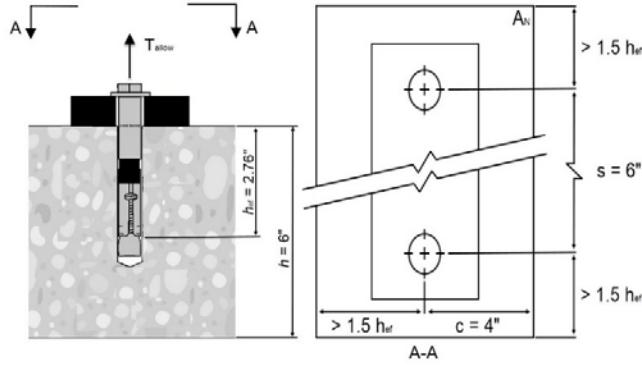


FIGURE 5—MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS

**Given:**  
 Two HSL4 M10 anchors under static tension load as shown.  
 $h_{ef} = 2.76$  in.  
 Normal weight concrete with  $f'_c = 3,000$  psi.  
 No supplementary reinforcement.  
 Condition B (ACI 318-19 17.5.3, ACI 318-14 17.3.3(c) or ACI 318-11 D.4.3(c))  
 Assume uncracked normal-weight concrete.



**Needed:**  
 Using Allowable Stress Design (ASD) Calculate the allowable tension load for this configuration.

Calculation per ACI 318-14 Chapter 17, ACI 318-11 Appendix D and this report.	ACI 318-19 Ref.	ACI 318-14 Ref.	ACI 318-11 Ref.	Report Ref.
Step 1. Calculate steel strength of anchor in tension $N_{sa} = nA_{se,N}f_{uta} = 2 \times 0.090 \times 116,000 = 20,880 \text{ lb}$	17.6.1.2	17.4.1.2	D.5.1.2	Table 2
Step 2. Calculate steel capacity $\Phi N_{sa} = 0.75 \times 20,880 = 15,600 \text{ lb}$	17.5.3	17.3.3(a)	D.4.3(a)	Table 2
Step 3. Calculate concrete breakout strength of anchor in tension $N_{cbg} = \frac{A_{Nc}}{A_{Nco}} \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b$	17.6.2.1	17.4.2.1	D.5.2.1	§4.1.3
Step 4. Verify minimum spacing and edge distance: Table 3 Case A: $h_{min} = 5-1/2 \text{ in.} < 6 \text{ in.}$ okay $\text{slope} = \frac{9.5 - 2.75}{2.75 - 5} = -3.0$ For $c_{min} = 4 \text{ in.}$ <input type="checkbox"/> $s_{min} = 9.5 - [(4 - 2.75)(-3.0)] = 5.75 \text{ in.} < 6 \text{ in.}$ <input type="checkbox"/> okay	17.9	17.7	D.8	Table 2
Step 5. Calculate $A_{Nco}$ and $A_{Nc}$ for the anchorage: $A_{Nco} = 9h_{ef}^2 = 9(2.76)^2 = 68.6 \text{ in}^2$ $A_{Nc} = (1.5h_{ef} + c)(3h_{ef} + s) = [1.5(2.76) + 4][3(2.76) + 6] = 116.2 \text{ in}^2 < 2A_{Nco} \therefore \text{okay}$	17.6.2.1	17.4.2.1	D.5.2.1	Table 2
Step 6. Calculate $N_b = k_{unscr} \lambda_a \sqrt{f'_c} h_{ef}^{1.5} = 24(1.0)\sqrt{3,000}(2.76)^{1.5} = 6,027 \text{ lb}$	17.6.2.2	17.4.2.2	D.5.2.2	Table 2
Step 7. Modification factor for eccentricity $\rightarrow$ no eccentricity $e'_N = 0 \therefore \Psi_{ec,N} = 1.0$	17.6.2.3.1	17.4.2.4	D.5.2.4	-
Step 8. Modification factor for edge $1.5h_{ef} = 1.5(2.76) = 4.13 \text{ in.} > c$ <input type="checkbox"/> $\Psi_{ed,N}$ must be calculated: $\Psi_{ed,N} = 0.7 + 0.3 \frac{4}{1.5(2.76)} = 0.99$	17.6.2.4.1	17.4.2.5	D.5.2.5	Table 2
Step 9. Modification factor for cracked concrete, $k = 24$ used in D.5.2.2 $\rightarrow \Psi_{c,N} = 1.0$ (see Step10)	17.6.2.5.1	17.4.2.6	D.5.2.6	Table 2
Step 10. Splitting Modification factor $\Psi_{cp,N} = \frac{\max  c  \square 1.5 h_{ef}}{c_{ac}} = \frac{1.5(2.76)}{4.375} = 0.94$	17.6.2.5.2	17.4.2.7	D.5.2.7	Table 3
Step 11. Calculate $N_{cbg} = \frac{116.2}{68.6} \times 1.0 \times 0.99 \times 1.0 \times 0.94 \times 6,027 = 9,500 \text{ lb}$	17.6.2.1	17.4.2.1	D.5.2.1	-
Step 12. Check pullout strength in Table 2 $\rightarrow N_{p,unscr}$ does not govern	17.6.3.2.1	17.4.3.2	D.5.3.2	§ 4.1.4 Table 2
Step 13. $\Phi N_{cbg} = 0.65 \times 9,500 = 6,175 \text{ lb} < \Phi N_s$ <input type="checkbox"/> $\Phi N_{cbg}$ controls	17.5.3	17.3.3(c)	D.4.3(c)	Table 2
Step 14. To convert to ASD, assume $U = 1.2D + 1.6L$ : $T_{allow} = \frac{6,175}{1.48} = 4,172 \text{ lb.}$	-	-	-	§ 4.2

FIGURE 6—EXAMPLE CALCULATION

**DIVISION: 03 00 00—CONCRETE**  
**Section: 03 16 00—Concrete Anchors**

**DIVISION: 05 00 00—METALS**  
**Section: 05 05 19—Post-Installed Concrete Anchors**

**REPORT HOLDER:**

HILTI, INC.

**EVALUATION SUBJECT:****HILTI HSL4 CARBON STEEL HEAVY DUTY EXPANSION ANCHORS FOR CRACKED AND UNCRACKED CONCRETE****1.0 REPORT PURPOSE AND SCOPE****Purpose:**

The purpose of this evaluation report supplement is to indicate that the Hilti HSL4 carbon steel heavy duty expansion anchors in cracked and uncracked concrete, described in ICC-ES evaluation report [ESR-4386](#), have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

**Applicable code editions:**

- 2020 *City of Los Angeles Building Code* (LABC)
- 2020 *City of Los Angeles Residential Code* (LARC)

**2.0 CONCLUSIONS**

The Hilti HSL4 carbon steel heavy duty expansion anchors, described in Sections 2.0 through 7.0 of the evaluation report [ESR-4386](#), comply with LABC Chapter 19, and LARC, and are subjected to the conditions of use described in this supplement.

**3.0 CONDITIONS OF USE**

The Hilti HSL4 carbon steel heavy duty expansion anchors described in this evaluation report supplement must comply with all of the following conditions:

- All applicable sections in the evaluation report [ESR-4386](#).
- The design, installation, conditions of use and identification of the anchors are in accordance with the 2018 *International Building Code*® (2018 IBC) provisions noted in the evaluation report [ESR-4386](#).
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16 and 17, as applicable.
- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.
- The allowable and strength design values listed in the evaluation report and tables are for the connection of the anchors to the concrete. The connection between the anchors and the connected members shall be checked for capacity (which may govern).
- For the design of wall anchorage assemblies to flexible diaphragms, the anchor shall be designed per the requirements of City of Los Angeles Information Bulletin P/BC 2020-071.

This supplement expires concurrently with the evaluation report, reissued March 2021 and revised July 2021.

**DIVISION: 03 00 00—CONCRETE**  
**Section: 03 16 00—Concrete Anchors**

**DIVISION: 05 00 00—METALS**  
**Section: 05 05 19—Post-Installed Concrete Anchors**

**REPORT HOLDER:**

HILTI, INC.

**EVALUATION SUBJECT:****HILTI HSL4 CARBON HEAVY DUTY EXPANSION ANCHORS FOR CRACKED AND UNCRACKED CONCRETE****1.0 REPORT PURPOSE AND SCOPE****Purpose:**

The purpose of this evaluation report supplement is to indicate that the Hilti HSL4 Carbon Steel Heavy Duty Expansion Anchors in cracked and uncracked Concrete, described in ICC-ES evaluation report ESR-4386, have also been evaluated for compliance with the codes noted below.

**Applicable code editions:**

- 2020 *Florida Building Code—Building*
- 2020 *Florida Building Code—Residential*

**2.0 CONCLUSIONS**

The Hilti HSL4 carbon steel anchors, described in Sections 2.0 through 7.0 of ICC-ES evaluation report ESR-4386, comply with the *Florida Building Code—Building* and the *Florida Building Code—Residential*, provided the design requirements are determined in accordance with the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable. The installation requirements noted in ICC-ES evaluation report ESR-4386 for the 2018 *International Building Code*® meet the requirements of the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable.

Use of the Hilti HSL4 carbon steel anchors have also been found to be in compliance with the High-Velocity Hurricane Zone provisions of the *Florida Building Code—Building* and the *Florida Building Code—Residential*, with the following conditions:

- a) For anchorage to wood members, the connection subject to uplift, must be designed for no less than 700 pounds (3114 N).
- b) For connection to aluminum members, all expansion anchors must be installed no less than 3 inches from the edge of concrete slab and/or footings. All expansion anchors shall develop an ultimate withdrawal resisting force equal to four times the imposed load, with no stress increase for duration of load.

For products falling under Florida Rule 61G20-3, verification that the report holder's quality-assurance program is audited by a quality-assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official, when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the evaluation report, reissued March 2021 and revised July 2021.